

ASSIGNMENT #7

1. The block is allowed to deform in y-direction
 $\therefore \epsilon_y = \Delta$; $\epsilon_x = \epsilon_z = 0$; Δ assumed deformation

From Stress Relationship:

$$\sigma_x - \nu(\sigma_y + \sigma_z) = E\epsilon_x = 0 \quad \dots (1)$$

$$\sigma_y - \nu(\sigma_x + \sigma_z) = E\epsilon_y = E\Delta \quad \dots (2)$$

$$\sigma_z - \nu(\sigma_x + \sigma_y) = E\epsilon_z = 0 \quad \dots (3)$$

$$\sigma_x(1+\nu) - \sigma_z(1+\nu) = 0 \quad \dots \text{Equating (1) and (3).}$$

$$\sigma_x = \sigma_z$$

From (2)

$$\sigma_y - \nu(2\sigma_x) = E\Delta$$

$$\sigma_x = -\frac{E\Delta}{2\nu} + \frac{\sigma_y}{2\nu} \quad \dots (4)$$

From (4) and (3) and equating $\sigma_x = \sigma_z$.

$$(1-\nu) \left[-\frac{E\Delta}{2\nu} + \frac{\sigma_y}{2\nu} \right] = \nu\sigma_y$$

Solving for σ_y :

$$\sigma_y \left[\nu - \frac{1-\nu}{2\nu} \right] = -\frac{1-\nu}{2\nu} E\Delta$$

$$\therefore \sigma_y = \frac{(1-\nu)}{(1+\nu)(1-2\nu)} \times E\Delta$$

in terms of F and a, L, C

$$\frac{F_0}{4a^2} = \frac{(1-\nu)}{(1+\nu)(1-2\nu)} \frac{E C}{L}$$

$$\therefore F_0 = \frac{(1-\nu)}{(1+\nu)(1-2\nu)} \frac{4Eca^2}{L}$$

$$2. \quad \sigma_x = 130 \text{ MPa} ; \quad \sigma_y = -70 \text{ MPa} ; \quad \tau_{xy} = 80 \text{ MPa}.$$

$$\nu = 0.3, \quad E = 205 \text{ GPa}.$$

$$\begin{aligned} \sigma_{1,2} &= \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} \\ &= \frac{130 - 70}{2} \pm \sqrt{\left(\frac{130 + 70}{2}\right)^2 + 80^2}. \end{aligned}$$

$$\sigma_{1,3} = 30 \pm 128.06$$

$$\sigma_1 = 158.06 \text{ MPa}.$$

$$\sigma_3 = -98.06 \text{ MPa}.$$

$$2. \quad \epsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)]$$

$$= \frac{1}{205 \times 10^3} [103 + 0.3 \times 70] = 736 \times 10^{-6}.$$

$$3. \quad \sigma_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)]$$

$$= \frac{1}{205 \times 10^3} [-70 - 0.3 \times 130] = -531 \times 10^{-6}$$

$$4. \quad \epsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)]$$

$$= \frac{1}{205 \times 10^3} [-0.3 \times (70 + 130)] = -87 \times 10^{-6}.$$

$$5. \quad E = 2G(1+\nu) \quad ; \quad G = \frac{E}{2(1+\nu)} = \frac{205 \times 10^3}{2 \times (1+0.3)}.$$

$$G = 78.84 \text{ GPa}.$$

$$\tau_{xy} = G \gamma_{xy}$$

$$\gamma_{xy} = \frac{\tau_{xy}}{G}$$

$$= \frac{80}{205 \times 10^3} = 1015 \times 10^{-6}$$

$$6. \quad \epsilon_{I, III} = \frac{\epsilon_x + \epsilon_y}{2} \pm \sqrt{\left(\frac{\epsilon_x - \epsilon_y}{2}\right)^2 + \left(\frac{\gamma_{xy}}{2}\right)^2}$$

$$= \frac{[736 - 531] \times 10^{-6}}{2} \pm 10^{-6} \sqrt{\left(\frac{736 + 531}{2}\right)^2 + \left(\frac{1015}{2}\right)^2}$$

$$= 102.5 \times 10^{-6} \pm 811.71 \times 10^{-6}$$

$$7. \quad \epsilon_I = 914.21 \times 10^{-6}$$

$$\epsilon_{III} = 709.21 \times 10^{-6}$$

8, 9 P = Internal Pressure.

$$\bar{\sigma}_\theta = \frac{pr}{t} \quad ; \quad \bar{\sigma}_z = \frac{pr}{2t} = \frac{\bar{\sigma}_\theta}{2}$$

$$\bar{\sigma}_r = 0$$

Max Shear Stress Criteria

$$\frac{\bar{\sigma}_{\max} - \bar{\sigma}_{\min}}{2} = \frac{Y}{2}$$

$$\therefore \bar{\sigma}_{\max} - \bar{\sigma}_{\min} = Y$$

$$\bar{\sigma}_{\max} = \bar{\sigma}_\theta \quad ; \quad \bar{\sigma}_{\min} = 0$$

$$\bar{\sigma}_\theta - 0 = Y$$

$$\bar{\sigma}_\theta = 250 \text{ MPa} \quad ; \quad \bar{\sigma}_z = \bar{\sigma}_\theta / 2$$

$$\bar{\sigma}_z = \frac{250}{2} = 125 \text{ MPa} \quad ; \quad \bar{\sigma}_r = 0$$

$$\epsilon_\theta = \frac{1}{E} [\bar{\sigma}_\theta - \nu (\bar{\sigma}_r + \bar{\sigma}_z)]$$

$$= \frac{1}{200 \times 10^3} [200 - 0.3 \times 125] = 1.0625 \times 10^{-3}$$

$$\epsilon_z = \frac{1}{E} [\bar{\sigma}_z - \nu (\bar{\sigma}_r + \bar{\sigma}_\theta)] = \frac{1}{200 \times 10^3} [125 - 0.3 \times 250]$$

$$= 0.25 \times 10^{-3}$$

$$\therefore \% \text{ Change in length} = \frac{\delta L}{L} \times 100\% = \epsilon_z \times 100$$

$$= 0.25 \times 10^{-3} \times 100 = 0.025\%$$

% Change in Circumference .

$$= \epsilon_0 \times 100$$

$$= 1.0625 \times 10^{-3} \times 100$$

$$= 0.106\%$$